Planning Proposal Proposed Residential Development

### Oakmont – Stage 2 28 Fairway Drive, Kellyville

TRAFFIC AND PARKING ASSESSMENT REPORT

23 October 2015

Ref 15550



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#### **Document Verification**

Location:	Oakmont – Stage 2	Job Numl	ber	15550	
	28 Fairway Drive, Kellyville				
Revision	Details	Prep	ared	Аррі	oved
E in al		By	Date	By	Date
Final	Final for Submission	СР	23/10/15	RV	23/10/15

# 1. INTRODUCTION

This report has been prepared to accompany a Planning Proposal for a residential development to be located at 28 Fairway Drive, Kellyville (Figures 1 and 2).

The Joint Regional Planning Panel has previously approved the staged construction of a new residential apartment development on the site, comprising a total of 172 apartments with atgrade and basement car parking (DA 824/2013/JP). Stage 1 is located on the eastern side of the riparian corridor and is currently under construction.

The North West Rail Link Corridor Strategy specifically identifies the subject site as a shortterm opportunity site, suitable for heights of between 7 and 12 storeys. As such, the Planning Proposal seeks to increase the maximum building height of Stage 2 to 12 storeys, yielding a potential total of approximately 240-250 apartments, compared with the previously approved 79 apartments.

Off-street parking for Stage 2 is to be provided in a new two-level basement car parking area, with vehicular access to be provided directly via Fairway Drive, as per the previous approval.

The purpose of this report is to assess the traffic and parking implications of the development proposal and to that end this report:

- describes the site and provides details of the Planning Proposal
- reviews the road network in the vicinity of the site, and the traffic conditions on that road network
- estimates the traffic generation potential of the Planning Proposal, and assigns that traffic generation to the road network serving the site
- assesses the traffic implications of the Planning Proposal in terms of road network capacity

• assesses the adequacy and suitability of the quantum of off-street car parking provided on the site.





## 2. PROPOSED DEVELOPMENT

#### Site

The subject site is located on the eastern side of Fairway Drive, approximately 150m south of Laura Street, and has a secondary frontage to Horatio Avenue. The site has street frontages approximately 74m in length to Fairway Drive, 185m in length to Horatio Avenue and occupies and area of approximately 20,237m<sup>2</sup>.

Stage 1 - i.e. east of the riparian corridor - is currently under construction.

Stage 2 - i.e. west of the riparian corridor - is currently occupied by the contractor's site office and amenities as well as used as a material storage area for Stage 1.

#### **Proposed Development**

The Planning Proposal seeks to increase the maximum building height of Stage 2 to 12 storeys, yielding a potential total of 240-250 apartments, compared with the previously approved 79 apartments.

Off-street parking for Stage 2 is to be provided in a new two-level basement car parking area, with vehicular access to be provided directly via Fairway Drive, as per the previous approval.

As part of the infrastructure upgrade within the precinct, Spurway Drive is to be extended to form a new T-junction proposed at Fairway Drive, where all turning movements will be permitted.

The proposed extension of Spurway Drive to Fairway Drive will comprise a 12.0 wide road pavement within a 20.0m wide road reservation in accordance with the requirements for an *enhanced collector road* in the Balmoral Road Release Area as specified in Part D of *The Hills DCP 2012*. That road pavement width will be capable of accommodating the introduction of new bus routes with bus stops at regular intervals on both sides of the road.

Loading/servicing for the proposed development is expected to be undertaken by a variety of vehicles up to and including medium and large rigid trucks such as garbage trucks and removalist trucks. All loading/unloading of trucks will take place from within indented onstreet parking bays and not within the basement car parking areas. Garbage trucks will approach the waste storage areas via access ways designed for this purpose and in accordance with Council's requirements.

Concept plans of the Planning Proposal development are reproduced in the following pages.

#### North West Rail Link Corridor Strategy

As described on the Transport for NSW website, the new North West Rail Link is Australia's largest public transport infrastructure project and is currently under construction, with opening scheduled for the first half of 2019.

The project will deliver eight new railway stations (including Norwest and Bella Vista Stations) and 4,000 commuter car parking spaces to Sydney's growing north-west where it is expected that an extra 200,000 people will live in the coming decades.

Train services will operate every four minutes, or at least 15 services per hour, during peak periods.

The subject site lies within the "Norwest" precinct as defined in the *North West Rail Link Corridor Strategy (September 2013)* document. The Strategy document indicates that the North West Rail Link will provide opportunities to increase residential densities within walking distance of Norwest Railway Station, including the subject site which will be approximately 8-10 minutes walking distance.

A Structure Plan for the Norwest Study Area is also reproduced in the following pages.



#### VARGA TRAFFIC PLANNING PTY LTD



#### VARGA TRAFFIC PLANNING PTY LTD





28 Fairway Drive, Kellyville



## 3. TRAFFIC ASSESSMENT

#### **Road Hierarchy**

The road hierarchy allocated to the surrounding road network by the Roads and Maritime Services is illustrated on Figure 3.

Windsor Road is classified by the RMS as a *State Road* and provides the key north-south road link in the area, linking North Parramatta to Kellyville Ridge. It typically carries two traffic lanes in each direction in the vicinity of the site, with additional lanes provided at key intersections to accommodate turning movements.

Norwest Boulevard is also classified by the RMS as a *State Road* and provides the key eastwest road link in the area, linking Windsor Road to Old Windsor Road and the M7 Motorway. It typically carries two traffic lanes in each direction in the vicinity of the site, with opposing traffic flows separated by a centre median island.

Fairway Drive and Solent Circuit are local, unclassified roads which perform the function of *collector routes* through the area, linking Windsor Road to Norwest Boulevard.

#### **Proposed Road Network Improvements**

Council is proposing to implement a number of substantial road network improvements to increase the capacity of Norwest Boulevard. These improvements are planned to be implemented over the next 5 years and will include:

- widening of Norwest Boulevard to 3 lanes in each direction, including a dedicated Bus Lane
- replacing all of the existing roundabouts on Norwest Boulevard with traffic signal controlled intersections
- provision of a new right-turn movement into Norwest Boulevard from Solent Circuit (west) in conjunction with the installation of traffic signals at that intersection.





The proposed road network improvements are intended to alleviate existing peak hour congestion on Norwest Boulevard as well as to cater for the future growth anticipated in the Norwest area. In addition, it is pertinent to note that the proposed improvements did not take into account the reduced traffic demands resulting from employees using the North West Rail Link. Completion of the North West Rail Link is expected to result in further improvements in traffic conditions in the area.

#### **Existing Traffic Controls**

The existing traffic controls which apply to the road network in the vicinity of the site are illustrated on Figure 4. Key features of those traffic controls are:

- a 70 km/h SPEED LIMIT which applies to Norwest Boulevard
- a 50 km/h SPEED LIMIT which applies to Solent Circuit, Fairway Drive and all other local roads in the area
- TRAFFIC SIGNALS in Norwest Boulevard where it intersects with Windsor Road and also Old Windsor Road
- a ROUNDABOUT in Solent Circuit where it intersects with Fairway Drive
- NO RIGHT-TURN restrictions in Fairway Drive turning onto Windsor Road during the morning and afternoon commuter peak periods.

#### **Existing Traffic Conditions**

An indication of the existing traffic conditions on the adjacent road network is provided by detailed peak period traffic surveys conducted as part of a recent traffic study undertaken for the adjacent site located at 47 Spurway Drive. The traffic surveys were conducted at the intersection of Solent Circuit, Fairway Drive and Hillsong site access during the morning and afternoon commuter peak periods and reveal that:

- two-way traffic flows in Fairway Drive are typically in the order of 500 vph during the *morning* commuter peak period, reducing to 250 vph during the *afternoon* commuter peak period
- two-way traffic flows in Solent Circuit are typically in the order of 400-600 vph during commuter peak periods.

#### **Projected Traffic Generation**

An indication of the traffic generation potential of the Planning Proposal is provided by reference to the Roads and Maritime Services publication *Technical Direction TDT 2013/04a* (*August 2013*) which notes that the Technical Direction *must* be followed when the RMS is undertaking trip generation assessments.

The *Technical Direction* is based on extensive surveys of a wide range of land uses and nominates the following traffic generation rates which are applicable to the Planning Proposal:

#### **High Density Residential Flat Buildings**

AM:	$0.19\ {\rm peak}\ {\rm hour}\ {\rm vehicle}\ {\rm trips}\ {\rm per}\ {\rm dwelling}$
PM:	0.15 peak hour vehicle trips per dwelling

Application of the above traffic generation rate to the *nett increase* of approximately 170 residential apartments as outlined in the Planning Proposal, yields an additional traffic generation potential of approximately 33 and 26 vph during the morning and afternoon commuter peak periods, respectively.

Furthermore, it is likely that the traffic generation potential of the proposed development will be slightly less than is suggested in the table above as the traffic generation rates nominated in the RMS's *Technical Direction*, because:

• the site is located in close proximity to Norwest Market Town which comprises an extensive range of shops and services that can be accessed from the site by walking

• a new underground railway station is proposed underneath the Norwest Boulevard and Brookhollow Avenue intersection, approximately 8 to 10 minutes walking distance from the site. The new railway station could be expected to reduce the need for private car ownership and usage, particularly during commuter peak periods.

Nevertheless, in order to provide a more rigorous assessment of the traffic implications of the Planning Proposal, it has been assumed that *all* of the projected future traffic flows of 33 AM & 26 PM peak hour vehicle trips will be new or *additional* to the existing traffic flows currently using the adjacent road network.

Those projected future traffic flows have been assigned to the surrounding road network as illustrated on Figure 5.

#### **Traffic Implications - Road Network Capacity**

The traffic implications of development proposals primarily concern the effects that any *additional* traffic flows may have on the operational performance of the nearby road network. Those effects can be assessed using the SIDRA program which is widely used by the RMS and many LGA's for this purpose. Criteria for evaluating the results of SIDRA analysis are reproduced in the following pages.

As mentioned in the foregoing, a detailed traffic analysis was undertaken as part of a traffic study undertaken for the adjacent site located at 47 Spurway Drive. The projected additional traffic generation associated with that development, in addition to the existing traffic volumes, has been adopted as a "base case" for the Planning Proposal assessment.

The results of the SIDRA capacity analysis "movement summaries" of the Solent Circuit and Fairway Drive intersection are reproduced in full in Appendix B and summarised on Table 3.1 below, revealing that:

 the Solent Circuit and Fairway Drive intersection is expected to operate at *Level of* Service "A" under the existing, previously approved and 47 Spurway Drive traffic demands, with total average vehicle delays in the order of 5-6 seconds/vehicle



TO	TAL
AM	РМ
7 IN	21 IN
26 OUT	5 OUT
33	26

**PROJECTED TRAFFIC ASSIGNMENT** FIGURE 5

• under the projected future traffic demands expected to be generated by the Planning Proposal, the Solent Circuit and Fairway Drive intersection will continue to operate at *Level of Service "A"*, with *zero* increase in average vehicle delays.

In summary therefore, it is clear that the additional traffic flows expected to be generated by the development proposal will not have any unacceptable traffic implications in terms of road network capacity, and will therefore not require any improvements or remedial treatments to increase the capacity of the nearby intersections.

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TABLE 3.1 - RESU SOLENT CIRC			DF	
Key Indicators	Traffic E Previously	sting Demand + Approval + vay Drive	Projected Proposal Tra	Planning affic Demand
	AM	PM	AM	PM
Level of Service	А	А	А	А
Degree of Saturation	0.590	0.401	0.605	0.408
Average Vehicle Delay (secs/veh)	6.0	5.0	6.0	5.0
	SOL	FAIP	SOL	FAIQ

# **Criteria for Interpreting Results of Sidra Analysis**

#### 1. Level of Service (LOS)

LOS	Traffic Signals and Roundabouts	Give Way and Stop Signs
'A'	Good operation.	Good operation.
'B'	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
'C'	Satisfactory.	Satisfactory but accident study required.
'D'	Operating near capacity.	Near capacity and accident study required.
'E'	At capacity; at signals incidents will cause excessive	At capacity and requires other control mode.
	delays. Roundabouts require other control mode.	
'F'	Unsatisfactory and requires additional capacity.	Unsatisfactory and requires other control mode.

#### 2. Average Vehicle Delay (AVD)

The AVD provides a measure of the operational performance of an intersection as indicated on the table below which relates AVD to LOS. The AVD's listed in the table should be taken as a guide only as longer delays could be tolerated in some locations (ie inner city conditions) and on some roads (ie minor side street intersecting with a major arterial route).

Level of Service	Average Delay per Vehicle (secs/veh)	Traffic Signals, Roundabout	Give Way and Stop Signs
А	less than 14	Good operation.	Good operation.
В	15 to 28	Good with acceptable delays and spare capacity.	Acceptable delays and spare capacity.
С	29 to 42	Satisfactory.	Satisfactory but accident study required.
D	43 to 56	Operating near capacity.	Near capacity and accident study required.
E	57 to 70	At capacity; at signals incidents will cause excessive delays. Roundabouts require other control mode.	At capacity and requires other control mode.

#### 3. Degree of Saturation (DS)

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The DS is another measure of the operational performance of individual intersections.

For intersections controlled by traffic signals<sup>1</sup> both queue length and delay increase rapidly as DS approaches 1, and it is usual to attempt to keep DS to less than 0.9. Values of DS in the order of 0.7 generally represent satisfactory intersection operation. When DS exceeds 0.9 queues can be anticipated.

For intersections controlled by a roundabout or GIVE WAY or STOP signs, satisfactory intersection operation is indicated by a DS of 0.8 or less.

The values of DS for intersections under traffic signal control are only valid for cycle length of 120 secs.

# 4. PARKING IMPLICATIONS

#### **Existing Parking Restrictions**

Given the semi-rural nature of Fairway Drive and the surrounding area, there are generally no kerbside parking restrictions which apply to the road network in the vicinity of the site.

#### **Off-Street Parking Provisions**

Whilst the precise details of the basement car parking layout is not yet known, it is envisaged that the proposed parking provision will be in accordance with the requirements specified in *State Environmental Planning Policy No* 65 – *Design Quality of Residential Flat Development (Amendment No 3), 2015.* 

The geometric design layout of the proposed vehicular access and car parking arrangements will ultimately be designed to comply with the relevant aspects specified in the Standards Australia publication *Parking Facilities Part 1: Off-Street Car Parking AS2890.1 - 2004* in respect of parking bay dimensions, aisle widths and ramp grades.

#### **Loading/Servicing Provisions**

The proposed development is expected to be serviced by a variety of commercial vehicles up to and including medium and large rigid garbage and removalist trucks.

It is therefore concluded that the Planning Proposal will not have any unacceptable parking or loading implications.

# **APPENDIX A**

# TRAFFIC SURVEY DATA

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0730 - 0745	7	7	54	4	9	0	1	0	0	1	11	0	94	0730 - 0745	0	0	0	0	0	0	0	0	0	0	0	0	0
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0815 - 0830	21	9	96	6	20	4	0	0	0	3	30	1	190	0815 - 0830	0	0	1	1	0	0	0	0	0	0	0	0	2
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1700 - 1715	3	2	12	60	53	9	2	8	2	7	50	13	221		1700 - 1715	0	0	0	0	0	0	0	0	0	0	0	0	0
1715 - 1730	2	5	11	42	42	13	4	2	1	6	48	9	185		1715 - 1730	0	0	0	0	0	0	0	0	0	0	0	0	0
1730 - 1745	6	4	8	34	33	3	1	4	2	4	39	7	145		1730 - 1745	0	0	0	0	0	0	0	0	0	0	0	0	0
1745 - 1800	1	7	9	28	33	5	4	2	2	2	31	10	134		1745 - 1800	0	0	0	0	0	0	0	0	0	0	0	0	0
1800 - 1815	2	6	13	27	36	3	1	5	1	5	22	7	128		1800 - 1815	0	0	0	0	0	0	0	0	0	0	0	0	0
1815 - 1830	2	9	8	29	24	1	1	4	0	5	44	4	131		1815 - 1830	0	0	0	0	0	0	0	0	0	0	0	0	0
Period End	26	36	93	300	289	38	18	30	8	32	306	68	1244		Period End	0	0	0	0	0	0	0	0	0	0	0	0	0
Lights	1	ORT	Н		WEST			SOUTI	4		EAST				<u>Heavies</u>	l	NORTH	Н		WEST	Г	;	SOUT	Н		EAST		
	Fa	airway	Dr	S	olent C	ct	Hills	ong Ac	cess	S	olent C	ct				F	airway	Dr	So	olent C	Cct	Hills	ong A	ccess	S	olent C	ct	
Peak Time	L	I	<u>R</u>	L	I	<u>R</u>	L	Ι	<u>R</u>	L	T	<u>R</u>	тот		Peak Time	L	<u>T</u>	<u>R</u>	Ŀ	<u>T</u>	<u>R</u>	L	T	<u>R</u>	L	I	<u>R</u>	тот
1630 - 1730	15	10	55	182	163	26	11	15	3	16	170	40	706		1630 - 1730	0	0	0	0	0	0	0	0	0	0	0	0	0
1645 - 1745	14	12	42	165	161	25	10	17	5	17	164	33	665		1645 - 1745	0	0	0	0	0	0	0	0	0	0	0	0	0
1700 - 1800	12	18	40	164	161	30	11	16	7	19	168	39	685		1700 - 1800	0	0	0	0	0	0	0	0	0	0	0	0	0
1715 - 1815	11	22	41	131	144	24	10	13	6	17	140	33	592		1715 - 1815	0	0	0	0	0	0	0	0	0	0	0	0	0
1730 - 1830	11	26	38	118	126	12	7	15	5	16	136	28	538		1730 - 1830	0	0	0	0	0	0	0	0	0	0	0	0	0
PEAK HOUR	15	10	55	182	163	26	11	15	3	16	170	40	706		PEAK HOUR	0	0	0	0	0	0	0	0	0	0	0	0	0
Combined	1	ORT	H		WEST		5	SOUTI	4		EAST		1		Peds	I	NORTH	H		WEST	Г		SOUT	H		EAST		
	Fa	airway	Dr	S	olent C	ct	Hills	ong Ac	cess	S	olent C	ct				F	airway	Dr	So	olent C	Cct	Hills	ong A	ccess	S	olent C	ct	
Time Per	L	I	<u>R</u>	L	I	<u>R</u>	L	I	<u>R</u>	L	I	<u>R</u>	тот		Time Per	UNC	LASSI	FIED	UNC	LASSI	FIED	UNC	LASSI	IFIED	UNC	LASSI	FIED	тот
1630 - 1645	7	2	21	51	35	4	2	2	0	3	45	14	186		1630 - 1645		0			2			2			0		4
1645 - 1700	3	1	11	29	33	0	3	3	0	0	27	4	114		1645 - 1700		0			1			0			0		1
1700 - 1715	3	2	12	60	53	9	2	8	2	7	50	13	221		1700 - 1715		0			0			1			0		1
1715 - 1730	2	5	11	42	42	13	4	2	1	6	48	9	185		1715 - 1730		0			1			2			0		3
1730 - 1745	6	4	8	34	33	3	1	4	2	4	39	7	145		1730 - 1745		0			1			5			0		6
1745 - 1800	1	7	9	28	33	5	4	2	2	2	31	10	134		1745 - 1800		0			0			1			0		1
1800 - 1815	2	6	13	27	36	3	1	5	1	5	22	7	128		1800 - 1815		0			0			5			0		5
1815 - 1830	2	9	8	29	24	1	1	4	0	5	44	4	131		1815 - 1830		0			0			3			0		3
Period End	26	36	93	300	289	38	18	30	8	32	306	68	1244		Period End		0			5			19			0		24
Combined	1	ORT	H		WEST			SOUTI	4		EAST				Peds	I	NORTI	H		WEST	Г		SOUT	H		EAST		
	Fa	airway	Dr	S	olent C	ct	Hills	ong Ac	cess	S	olent C	ct				F	airway	Dr	So	olent C	Cct	Hills	ong A	ccess	S	olent C	ct	
Peak Time	L	I	<u>R</u>	L	I	<u>R</u>	L	I	<u>R</u>	L	I	<u>R</u>	тот		Peak Per	UNC	LASSI	FIED	UNC	LASSI	FIED	UNC	LASSI	IFIED	UNC	LASSI	FIED	тот
1630 - 1730	15	10	55	182	163	26	11	15	3	16	170	40	706		1630 - 1730		0			4			5			0		9
1645 - 1745	14	12	42	165	161	25	10	17	5	17	164	33	665		1645 - 1745		0			3			8			0		11
1700 - 1800	12	18	40	164	161	30	11	16	7	19	168	39	685		1700 - 1800		0			2			9			0		11
1715 - 1815	11	22	41	131	144	24	10	13	6	17	140	33	592		1715 - 1815		0			2			13			0		15
1730 - 1830	11	26	38	118	126	12	7	15	5	16	136	28	538		1730 - 1830		0			1			14			0		15
PEAK HOUR	15	10	55	182	163	26	11	15	3	16	170	40	706	3	PEAK HR		0			4			5			0		9

R	<b>R.O.A.</b>	r da	TA										Cli	ent	: Varga Tra	affic Planning				
	Reliable, O	riginal a	& Auther	ntic Re	esults	5							Job No/	Name	: 4422 BAL	JLKHAM HILLS	Spurway Dr			
DA	Ph.8819684	47, Fax 8	88196849	9, Mot	b. 041	8 239	019						Day/	/Date	: Monday 3	rd December	2012			
														N						
	Hours	s 1	Hours 2		Hou	irs 3		Hours 4		Ho	ours 5		-	AN AN	-					
										_				V						
				Fa	irway	' Dr										Fa	irway Dr			
	<u>PM PEA</u>	K		T		0										<b></b>		тот	AL VO	
	1630 - 17	30		237		80												F	OR PEF	RIOD
			0	237	0	80	0									398	0		COUNT	ED
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			55		10		15									0	155			
	Solen	t Cct			•												•			
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	0 182							4			0			0	627	627	• 0	323	323	
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													-	417	417	0	406	406		0
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	<b>R.O</b> .	A.R.	DA	ТА										CI	lient	: Varga Tr	affic Pla	anning					
	Reliab	le, Or	iginal &	& Auth	nentic F	Result	s							Job N	o/Name	: 4422 BA	ULKHAI	M HILL	S Spur	way Di			
DN	Ph.88	196847	7, Fax 8	381968	349, Mo	ob.041	8-239019							Day	/Date	: Monday	3rd Dec	ember	2012				
Lights	NOF	RTH	WE	ST	SOL	UTH		Heavies	NO	RTH	WE	ST	SO	UTH		Combined		RTH	T	ST	SOL	JTH	
	Winds	or Rd	Spurw	ay Dr	Winds	sor Rd			Winds	or Rd	Spurv	vay Dr	Winds	sor Rd			Wind	sor Rd	Spurv	vay Dr	Winds	or Rd	
Time Per	T	R	L	<u>R</u>	L	T	TOT	Time Per	I	R	L	R	L	T	тот	Time Pe	r <u>T</u>	R	L	R	L	I	тот
0700 - 0715	537		4		11	159	711	0700 - 0715	8		0		0	5	13	0700 - 071	5 545	0	4	0	11	164	724
0715 - 0730	493		3		5	132	633	0715 - 0730	6		0		0	5	11	0715 - 073	0 499	0	3	0	5	137	644
0730 - 0745	538		7		9	137	691	0730 - 0745	6		0		0	7	13	0730 - 074	5 544	0	7	0	9	144	704
0745 - 0800	567		6		6	189	768	0745 - 0800	5		0		0	6	11	0745 - 080	0 572	0	6	0	6	195	779
0800 - 0815	576		5		6	154	741	0800 - 0815	9		0		0	3	12	0800 - 081	5 585	0	5	0	6	157	753
0815 - 0830	554		5		8	202	769	0815 - 0830	1		0		0	9	10	0815 - 083	0 555	0	5	0	8	211	779
0830 - 0845	539		8		6	148	701	0830 - 0845	3		0		0	2	5	0830 - 084	5 542	0	8	0	6	150	706
0845 - 0900	491		7		7	167	672	0845 - 0900	3		0		0	10	13	0845 - 090	0 494	0	7	0	7	177	685
Per End	4295	0	45	0	58	1288	5686	Per End	41	0	0	0	0	47	88	Per Enc	4336	0	45	0	58	1335	5774
1.1.1.1.1	NO		14/***	OT	001			Line 1:	N.C.		14/7	0.7		1711		O any litte		DTU	14/7	OT		1711	1
Lights	NOF		WE	-		UTH		<u>Heavies</u>		RTH		ST		UTH		Combined	· · · · · · · · · · · · · · · · · · ·	RTH	WE	-	SOL		l
Peak Per	Winds		Spurw	-	Winds		TOT	Deals Dea	Winds	or Rd	Spurv	vay Dr	Winds	SOR Rd	TOT	De els De		sor Rd	Spurv	vay Dr	Winds		тот
	<u> </u>	<u>R</u>		<u>R</u>		T	тот	Peak Per	<u> </u>	<u>R</u>	<u> </u>	<u>R</u>		<u> </u>	тот	Peak Pe		R		<u>R</u>	L	T	
0700 - 0800	2135	0	20	0	31	617	2803	0700 - 0800	25	0	0	0	0	23	48	0700 - 080	_	0	20	0	31	640	2851
0715 - 0815	2174	0	21	0	26	612	2833	0715 - 0815	26	0	0	0	0	21	47	0715 - 081		0	21	0	26	633	2880
0730 - 0830	2235	0	23	0	29	682	2969	0730 - 0830	21	0	0	0	0	25	46	0730 - 083		0	23	0	29	707	3015
0745 - 0845	2236	0	24	0	26	693	2979	0745 - 0845	18	0	0	0	0	20	38	0745 - 084		0	24	0	26	713	3017
0800 - 0900	2160	0	25	0	27	671	2883	0800 - 0900	16	0	0	0	0	24	40	0800 - 090	0 2176	0	25	0	27	695	2923
PEAK HR	2236	0	24	0	26	693	2979	PEAK HR	18	0	0	0	0	20	38	PEAK HI	2254	0	24	0	26	713	3017
Peds	NOF	RTH	WE	ST	SOL	UTH											Windso	r Rd					
Time Per	Winds	or Rd	Spurw	vay Dr	Winds	sor Rd	TOT	Hours 1											18		N		
0700 - 0715	C	)	C	)	(	0	0				AME	PEAK							2236		· A -		
0715 - 0730	C	)	C	)	(	0	0	Hours 2			0745	- 0845				737	0	18	2254	-		>	
0730 - 0745	C	)	2	2	(	0	2									717	0	2236			4		
0745 - 0800	C	)	C	)	(	0	0	Hours 3								20	0	2254	•		,		
0800 - 0815	C	)	C	)	(	0	0					Spurv	vay Dr										
0815 - 0830	C	)	C	)	(	0	0	Hours 4			0	24	24			▲ ◄							
0830 - 0845	C	)	C	)	(	0	0						0	24	24 —								
0845 - 0900	C	)	0		(	0	0	Hours 5										. A					
Per End	(	)	2	2	(	0	2											n a					
													0	0	0 —								
	NOF	RTH	WE	ST	SOL	UTH					◀	- 26	26	0		▲		<b>▲</b>					
Peak Per	Winds	or Rd	Spurw	vay Dr	Winds	sor Rd	TOT												18				
0700 - 0800	C	)	2	2	(	0	2										26	713	2236				
0715 - 0815	C	)	2	2	(	0	2				©	Copyrig	ht ROAF	R DATA		739	26	693	2254				
0730 - 0830	C	)	2	2	(	0	2									719	0	20					
0745 - 0845	C	)	C	)	(	0	0									20			•				
0800 - 0900	C	)	C	)	(	0	0										Windso	r Rd					

### **APPENDIX B**

# SIDRA TRAFFIC MODELLING RESULTS

# Site: Existing + 47 Spurway Dr + Previous Approval AM

Solent Cct & Fairway Dr, Baulkham Hills Roundabout

Move	ment P <u>er</u>	formance - V	ehicle <u>s</u>								
Mov	OD	Demand	Flows	Deg.	Average	Level of	95% Back	of Queue	Prop.	Effective	Average
ID	Mov	Total	ΗV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
0 "		veh/h	%	v/c	sec		veh	m		per veh	km/h
South: Hillsong Access (S)											
1	L2	3	0.0	0.006	6.1	LOS A	0.0	0.2	0.63	0.55	45.5
2	T1	1	0.0	0.006	6.1	LOS A	0.0	0.2	0.63	0.55	46.5
3	R2	1	0.0	0.006	10.4	LOS A	0.0	0.2	0.63	0.55	46.6
Appro	ach	5	0.0	0.006	7.0	LOS A	0.0	0.2	0.63	0.55	45.9
East:	Solent Cct (	E)									
4	L2	7	0.0	0.162	6.1	LOS A	0.9	6.6	0.64	0.68	45.2
5	T1	90	0.0	0.162	6.0	LOS A	0.9	6.6	0.64	0.68	46.1
6	R2	42	0.0	0.162	10.3	LOS A	0.9	6.6	0.64	0.68	46.2
Appro	ach	139	0.0	0.162	7.3	LOS A	0.9	6.6	0.64	0.68	46.1
North:	Fairway Dr	r (N)									
7	L2	310	0.3	0.590	3.6	LOS A	5.5	38.3	0.38	0.53	45.6
8	T1	36	0.0	0.590	3.6	LOS A	5.5	38.3	0.38	0.53	46.6
9	R2	505	0.4	0.590	7.9	LOS A	5.5	38.3	0.38	0.53	46.7
Appro	ach	851	0.4	0.590	6.2	LOS A	5.5	38.3	0.38	0.53	46.3
West:	Solent Cct	(W)									
10	L2	64	4.7	0.105	3.2	LOS A	0.6	4.3	0.18	0.39	47.2
11	T1	69	0.0	0.105	3.1	LOS A	0.6	4.3	0.18	0.39	48.3
12	R2	11	0.0	0.105	7.4	LOS A	0.6	4.3	0.18	0.39	48.4
Appro	ach	144	2.1	0.105	3.5	LOSA	0.6	4.3	0.18	0.39	47.8
All Ve	hicles	1139	0.5	0.590	6.0	LOS A	5.5	38.3	0.39	0.53	46.5

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# Site: Existing + 47 Spurway Dr + Previous Approval PM

Solent Cct & Fairway Dr, Baulkham Hills Roundabout

Move	ment Perf	ormance - V	ehicles								
Mov	OD	Demand		Deg.	Average	Level of	95% Back		Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South	: Hillsong A	veh/h	%	v/c	sec		veh	m		per veh	km/h
	L2	. ,	0.0	0.030	4.0	LOS A	0.2	1.1	0.51	0.50	46.0
1		11	0.0		4.9		0.2		0.51	0.52	46.2
2	T1	15	0.0	0.030	4.9	LOS A	0.2	1.1	0.51	0.52	47.2
3	R2	3	0.0	0.030	9.1	LOS A	0.2	1.1	0.51	0.52	47.3
Appro	ach	29	0.0	0.030	5.3	LOSA	0.2	1.1	0.51	0.52	46.8
East: \$	Solent Cct (	E)									
4	L2	16	0.0	0.261	3.7	LOS A	1.6	11.1	0.35	0.52	45.9
5	T1	170	0.0	0.261	3.7	LOS A	1.6	11.1	0.35	0.52	46.9
6	R2	145	0.0	0.261	8.0	LOS A	1.6	11.1	0.35	0.52	47.0
Appro	ach	331	0.0	0.261	5.5	LOSA	1.6	11.1	0.35	0.52	46.9
North:	Fairway Dr	- (N)									
7	L2	97	0.0	0.179	4.0	LOS A	1.0	7.2	0.40	0.56	45.8
8	T1	10	0.0	0.179	3.9	LOS A	1.0	7.2	0.40	0.56	46.8
9	R2	99	0.0	0.179	8.2	LOS A	1.0	7.2	0.40	0.56	46.9
Appro	ach	206	0.0	0.179	6.0	LOSA	1.0	7.2	0.40	0.56	46.4
West:	Solent Cct	(W)									
10	L2	313	0.0	0.401	4.0	LOS A	2.8	19.6	0.44	0.50	46.6
11	T1	163	0.0	0.401	4.0	LOS A	2.8	19.6	0.44	0.50	47.6
12	R2	26	0.0	0.401	8.3	LOS A	2.8	19.6	0.44	0.50	47.7
Appro	ach	502	0.0	0.401	4.2	LOS A	2.8	19.6	0.44	0.50	47.0
All Vel	nicles	1068	0.0	0.401	5.0	LOS A	2.8	19.6	0.41	0.52	46.9

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akçelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# Site: Existing + 47 Spurway Dr + Previous Approval + Planning Proposal AM

Solent Cct & Fairway Dr, Baulkham Hills Roundabout

Move	ment Perfe	ormance - V	/ehicles								
Mov	OD	Demand Flows		Deg.	Average	Level of	95% Back		Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
Cauth		veh/h	%	v/c	sec		veh	m		per veh	km/h
	: Hillsong Ac	. ,									
1	L2	3	0.0	0.006	6.2	LOS A	0.0	0.2	0.64	0.55	45.5
2	T1	1	0.0	0.006	6.2	LOS A	0.0	0.2	0.64	0.55	46.4
3	R2	1	0.0	0.006	10.5	LOS A	0.0	0.2	0.64	0.55	46.6
Appro	ach	5	0.0	0.006	7.1	LOS A	0.0	0.2	0.64	0.55	45.9
East:	Solent Cct (I	E)									
4	L2	7	0.0	0.166	6.1	LOS A	1.0	6.8	0.65	0.69	45.1
5	T1	90	0.0	0.166	6.1	LOS A	1.0	6.8	0.65	0.69	46.1
6	R2	44	0.0	0.166	10.4	LOS A	1.0	6.8	0.65	0.69	46.2
Appro	ach	141	0.0	0.166	7.5	LOS A	1.0	6.8	0.65	0.69	46.1
North:	Fairway Dr	(N)									
7	L2	324	0.3	0.605	3.6	LOS A	5.7	40.3	0.39	0.53	45.6
8	T1	36	0.0	0.605	3.6	LOS A	5.7	40.3	0.39	0.53	46.6
9	R2	513	0.4	0.605	7.9	LOS A	5.7	40.3	0.39	0.53	46.7
Appro	ach	873	0.3	0.605	6.1	LOS A	5.7	40.3	0.39	0.53	46.3
West:	Solent Cct (	(W)									
10	L2	66	4.5	0.107	3.2	LOS A	0.6	4.4	0.19	0.39	47.2
11	T1	69	0.0	0.107	3.1	LOS A	0.6	4.4	0.19	0.39	48.3
12	R2	11	0.0	0.107	7.4	LOS A	0.6	4.4	0.19	0.39	48.4
Appro	ach	146	2.1	0.107	3.5	LOS A	0.6	4.4	0.19	0.39	47.8
All Ve	hicles	1165	0.5	0.605	6.0	LOS A	5.7	40.3	0.40	0.53	46.4

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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# Site: Existing + 47 Spurway Dr + Previous Approval + Planning Proposal PM

Solent Cct & Fairway Dr, Baulkham Hills Roundabout

Move	ment Per	ormance - V	ehicle <u>s</u>								
Mov	OD	Demand		Deg.	Average	Level of	95% Back		Prop.	Effective	Average
ID	Mov	Total	HV	Satn	Delay	Service	Vehicles	Distance	Queued	Stop Rate	Speed
South		veh/h	%	v/c	sec		veh	m	_	per veh	km/h
	: Hillsong A		0.0	0.000	4.0		0.0		0.54	0.50	40.0
1	L2	11	0.0	0.030	4.9	LOS A	0.2	1.1	0.51	0.53	46.2
2	T1	15	0.0	0.030	4.9	LOS A	0.2	1.1	0.51	0.53	47.2
3	R2	3	0.0	0.030	9.2	LOS A	0.2	1.1	0.51	0.53	47.3
Appro	ach	29	0.0	0.030	5.3	LOS A	0.2	1.1	0.51	0.53	46.8
East: \$	Solent Cct (	E)									
4	L2	16	0.0	0.266	3.7	LOS A	1.6	11.4	0.36	0.52	45.9
5	T1	170	0.0	0.266	3.7	LOS A	1.6	11.4	0.36	0.52	46.9
6	R2	150	0.0	0.266	8.0	LOS A	1.6	11.4	0.36	0.52	47.0
Appro	ach	336	0.0	0.266	5.6	LOS A	1.6	11.4	0.36	0.52	46.9
North:	Fairway Dr	- (N)									
7	L2	100	0.0	0.183	4.0	LOS A	1.1	7.4	0.40	0.57	45.8
8	T1	10	0.0	0.183	3.9	LOS A	1.1	7.4	0.40	0.57	46.8
9	R2	101	0.0	0.183	8.2	LOS A	1.1	7.4	0.40	0.57	46.9
Appro	ach	211	0.0	0.183	6.0	LOS A	1.1	7.4	0.40	0.57	46.4
West:	Solent Cct	(W)									
10	L2	319	0.0	0.408	4.0	LOS A	2.9	20.1	0.45	0.50	46.6
11	T1	163	0.0	0.408	4.0	LOS A	2.9	20.1	0.45	0.50	47.6
12	R2	26	0.0	0.408	8.3	LOS A	2.9	20.1	0.45	0.50	47.7
Appro	ach	508	0.0	0.408	4.2	LOS A	2.9	20.1	0.45	0.50	47.0
All Vel	nicles	1084	0.0	0.408	5.0	LOS A	2.9	20.1	0.41	0.52	46.8

Level of Service (LOS) Method: Delay (RTA NSW).

Vehicle movement LOS values are based on average delay per movement

Intersection and Approach LOS values are based on average delay for all vehicle movements.

Roundabout Capacity Model: SIDRA Standard.

SIDRA Standard Delay Model is used. Control Delay includes Geometric Delay.

Gap-Acceptance Capacity: SIDRA Standard (Akcelik M3D).

HV (%) values are calculated for All Movement Classes of All Heavy Vehicle Model Designation.

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